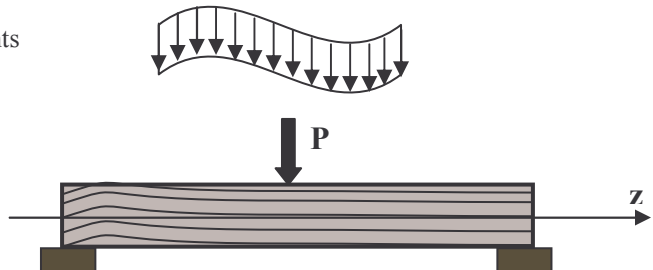
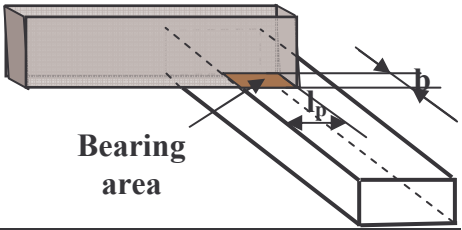
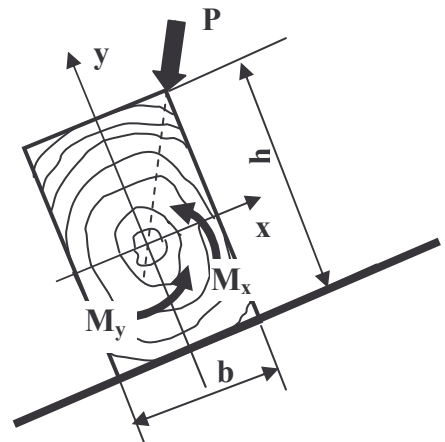




## TIMBER CONSTRUCTION

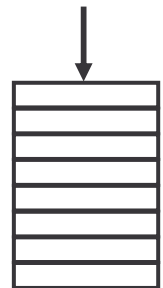
1. Design of timber elements is done according to Romanian Code NP005-96 using the following method:
- |  |                                      |                 |
|--|--------------------------------------|-----------------|
| a. allowable strength design method (ASDM) | b. limit states design method (LSDM) | c. both methods |
|--|--------------------------------------|-----------------|
2. According to limit states design method of timber member with simple cross-section, for „i” yield stress „ $m_{Ti}$ ” represents:
- |                                  |                                      |                          |
|----------------------------------|--------------------------------------|--------------------------|
| a. coefficient of side stability | b. coefficient of working conditions | c. treatment coefficient |
|----------------------------------|--------------------------------------|--------------------------|
3. Design resistance of a timber beam subjected to “i” stress represents:
- |  |   |  |
|--|---|--|
| a. The maximum stress which is taken by beam | b. The stress of the specific cross-section having the maximum load | c. The stress of the cross-section situated on middle of the beam length |
|--|---|--|
4. The design resistance (LSDM) of timber elements subjected to bending is evaluated using:
- 
- |                             |                                 |  |
|-----------------------------|---------------------------------|--|
| a. $M_r = R_i^c W_n m_{Ti}$ | b. $M_r = \sigma_{\max,ef} W_n$ | d. $M_r = \tau_{\max,ef} \frac{bI}{S}$ |
|-----------------------------|---------------------------------|--|
5. The bearing area is evaluated using the bearing coefficient,  $m_r$ , which has the following values:
- 
- |                |                            |              |
|----------------|----------------------------|--------------|
| a. $m_r < 1,0$ | b. $1,0 \leq m_r \leq 2,0$ | c. $m_r = 0$ |
|----------------|----------------------------|--------------|
6. The final maximum deflection of bending elements is evaluated with:
- |                               |   |  |
|-------------------------------|---|--|
| a. $f_{\max,ef} \leq f_{adm}$ | b. $f_{\max,final} = f_1 + f_2 + f_3 - f_c$ | c. $f_{\max,ef} = \frac{1}{250} L$<br>where L is the beam span |
|-------------------------------|---|--|
7. The maximum deflections under a biaxial bending is given by the following loads combination:
- |   |  |                      |
|---|--|----------------------|
| a. dead load + variable + joint deformation - precamber | b. dead load + variable load + precamber | c. joint deformation |
|---|--|----------------------|
8. Which of the following elements is subjected to bending:
- |                     |                    |                        |
|---------------------|--------------------|------------------------|
| a. rafter and joist | b. purlin and arch | c. wall plate and post |
|---------------------|--------------------|------------------------|
9. Which element is subjected primarily to biaxial bending?
- |           |                                       |                                      |
|-----------|---------------------------------------|--------------------------------------|
| a. rafter | b. purlin vertical to horizontal plan | c. purlin vertical to the roof slope |
|-----------|---------------------------------------|--------------------------------------|

10. Timber beams are subjected to biaxial bending and their verification equation is:



a.  $\sigma_{\max,ef} = \frac{(M_x)_{\max}}{W_{xn}} + \frac{(M_y)_{\max}}{W_{yn}}$     b.  $\sigma_{\max,ef} \leq \sigma_{ai}$     c.  $\pm \frac{(M_x)_{\max}}{(M_x)_r} \pm \frac{(M_y)_{\max}}{(M_y)_r} \leq 1$

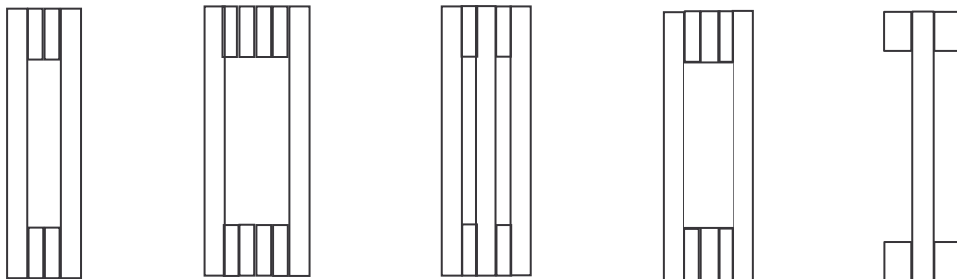
11. Glulam beams have design resistance higher than solid timber beam due to:



- a. the adhesive presence increases the shear behavior and the material is optimized by dispersing the strength-reducing defects in the laminating material throughout the member;
- b. net area of highly stress cross-section in the case of glulam member is larger than that of solid timber in the same location;
- c. high-quality laminations are located in that portion of the cross-section, which is more highly stressed;

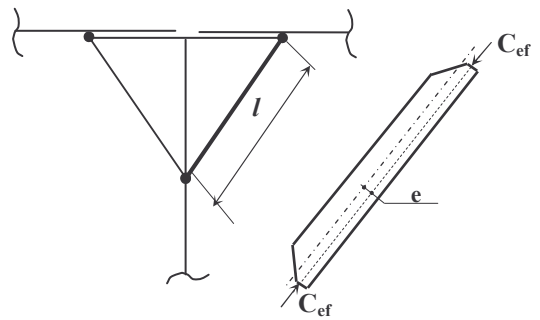
The following cross-section is representative for plywood web beams. For design resistance are used the following mechanical characteristics of:

12.



- a. the flanges material
- b. the web material
- c. both types of material proportional to their influence

The verification of the bracing from the attached figure is evaluated with:



13.

- a.  $C_{ef} / \varphi A \pm M_{ef}^t / W \leq R_{cl}^c$       b.  $C_{ef} / \varphi C_r \pm M_{ef}^t / M_r = 1$       c.  $-C_{ef} / C_r \pm M_{ef}^t / M_r \leq 1$

14.

For the short beam or beams subjected in transverse direction by the compression force  $C_{ef}$ , the stress evaluation is done with:

- a.  $\sigma = \frac{C_{ef}}{m_T A_{calcul}}$       b.  $\sigma = \frac{C_{ef} \lambda^2}{3100 A_{net}}$       c.  $\sigma = \frac{C_{ef}}{\varphi A}$

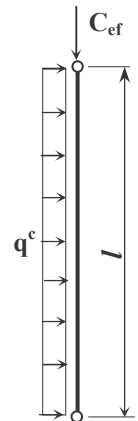
15.

Design resistance of an axial compression member is established with:

- a.  $C_r \leq R_{cl}^c A_{net}$       b.  $C_r = R_{cl}^c A_{calcul} \varphi m^c_T$       c.  $C_r \geq R_{cl}^c A$

16.

For solid timber column from the attached figure the verification of maximum stress is given by:



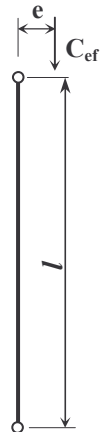
- a.  $-C_{ef} / C_r \pm M_{ef}^t / M_r \leq 1$       b.  $-C_{ef} / C_r \pm M_{ef}^t / M_r > 1$       c.  $C_{ef} / \varphi A_{calcul} \pm M_{ef}^t / W_{calcul} \leq R_{cl}^c$

17.

The equation  $\varphi = \frac{3100}{\lambda^2}$  defines:

- a. working coefficient      b. moisture content variation coefficient during construction service      c. buckling coefficient

Timber elements from the figure are subjected to:



18.

- a. biaxial bending                      b. compression and bending                      c. axial compression

19. The strength condition for a round wood column of 3,0 m length subjected to axial compression force  $C_{ef}$ , is given by the equation:

- a.  $\sigma_{ef} = \frac{C_{ef}}{\lambda^2 A_{brut}} \leq R_{c||}^c$                       b.  $\sigma_{ef} = \frac{C_{ef}}{\varphi A_{net} m^c_T} \leq R_{c||}^c$                       c.  $\sigma_{ef} = \frac{C_{ef}}{A} \leq R_{c||}^c$

20. A timber column subjected to buckling from compression is working in elastic domain when the slenderness ratio is:

- a.  $\lambda = 75$                       b.  $\lambda > 75$                       c.  $\lambda < 75$

21. The effective (buckling) length,  $l_f$ , of a timber column subjected to axial compression depends on:

- a. column length                      b. intensity of compression force                      c. type of end joints

22. The design resistance of a timber column subjected to eccentric compression force is given by:

- a.  $C_r = R_{c||}^c A_{calcul} \varphi m^c_T$                       b.  $C_r = R_{c||}^c A_{brut} m^c_T$                       c.  $C_r = R_{c||}^c W_{calcul} \varphi$

23. For solid timber column subjected to eccentric compression the load capacity is verified with the relation:

- a.  $\frac{C_{ef}}{\varphi C_r} + \frac{M_{ef}^f}{M_r} = 1$                       b.  $-\frac{C_{ef}}{C_r} \pm \frac{M_{ef}^f}{M} \leq 1,0$                       c.  $\frac{C_{ef}}{\varphi A_{calcul}} \pm \frac{M_{ef}^f}{W_{calcul}} \leq R_{c||}^c$

24. For a double hinge column, the buckling length is:

- a.  $l_f = 0,65 l$                       b.  $l_f = 1,20 l$                       c.  $l_f = l$

25. In the case of a column subjected to eccentric compression, the strength verification is done taking into account the axial compression only if:

- a.  $M_{ef}^f / W_{calcul} = 0,1 C_{ef} / A_{calcul}$                       b.  $M_{ef}^f / W_{calcul} > 10 \% C_{ef} / A_{calcul}$                       c.  $M_{ef}^f / W_{brut} \leq 10 \% C_{ef} / A_{brut}$

26. For top chord of a timber truss, the buckling length is:

- a.  $l_f = l$  – the member length between the theoretical end nodes                      b.  $l_f = 0,56 l$                       c.  $l_f = 1,5 l$

27. For a compound beam, the design resistance is given by  $T_{r,i} = R_t^c \cdot A_{net,i} \cdot m_{T,i} \cdot m_R$ , where  $m_R$  represents:

- a. coefficient of characteristic strength                      b. distribution coefficient of loads between wood components of the compound beam                      c. coefficient of technological processes applied during the element fabrication

